# The Absolute Node Coordinates Method of Large Deformation of the Flexible Beam System

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### Abstract

The application of an absolute nodal coordinate formulation (ANCF) in the coupling dynamics of flexible beams with large deformation was investigated. By using geometric nonlinear formulation and finite element method, the dynamics equations of planar beams system were developed by using the ANCF. The comparison between the ANC formulation and nonlinear strain-displacement formulation in the case of small deformation and large deformation indicates that the ANC formulation is more precise and suited for the beams with large deformation. The present results are verified by using the energy conservation law.

Keywords: Planar Beams System; Kinetics Equations; Absolute Nodal Coordinate Formulation; Large Deformation

As new materials and new technology application in the engineering, manifest the flexible deformation of motion mechanism, the motion mechanism for a wide range of motion or the applied load, large deformation phenomenon in a flexible body, manifests the geometrical nonlinear behavior. For example, dozens of meters long robotic arm on the international space station and large deformation problems of large area solar panels. In recent years, some scholars to work on a wide range of motion of geometrical nonlinear problem of a cantilever beam was studied<sup>[1, 2]</sup>, Based on the assumption of small deformation, and established an approximation of the coupled dynamic equations, however, due to ignore the higher order term in the dynamics equation, unable to solve the problem of large deformation the coupled dynamics of a cantilever beam. In order to establish a precise dynamic model of Shabana<sup>[3]</sup> proposed the absolute coordinate method, the characteristics of this method is an absolute displacement expression is simpler, quality array for constant array, simplifies the system model to a certain extent. And, more importantly, in the process of establishing dynamics equations, based on the geometric nonlinear theory, retained the longitudinal strain and elastic force of higher order term. Then, Shabana<sup>[4 ~ 6]</sup> the absolute coordinates method is applied to large rotation and large deformation of a cantilever beam. In this paper, the absolute coordinates method to plane beam system, starting from the exact expression of axial strain and curvature, the establishment of a flexible beam finite element discrete dynamics equation, on this basis, according to the constraint relation between flexible body movement, the establishment of a flexible beam system dynamics equation. Numerical simulation of flexible pendulum and double pendulum and demonstrate the validity of the modeling theory of absolute node coordinates.

## 1. The Dynamic Equation of Single Beam

In this paper, the research object of plane beam system, using Euler Bernoulli assumption, without considering shear deformation, that the deformation of beam cross section remains for plane and vertical axis. Establish  $Ox_0y_0$  absolute coordinate system, as shown in figure 1.For plane beam system, including the I beam, the beam is divided into several units,  $O_ex_ey_e$  is consolidation in the beam end unit of floating coordinate system.



Fig 1: Beam Element before and After Deformation

As shown in figure 1, r for arbitrary point on the beam axis P about absolute coordinates the coordinates of the matrix, can be expressed as:

$$r = \begin{bmatrix} r_1 \\ r_2 \end{bmatrix} = Se \tag{1}$$

 $r_1$ ,  $r_2$ , respectively is the absolute displacement in  $x_0$ ,  $y_0$  direction; S is the shape function; E array as a unit node coordinates.

$$e = \begin{bmatrix} e_{1} & e_{2} & e_{3} & e_{4} & e_{5} & e_{6} & e_{7} & e_{8} \end{bmatrix}^{T}$$
  
Absolute displacement of node:  
$$e_{1} = r_{1} \Big|_{x=0}, \quad e_{2} = r_{2} \Big|_{x=0}$$
  
$$e_{5} = r_{1} \Big|_{x=l}, \quad e_{6} = r_{2} \Big|_{x=l}$$
  
$$s_{1} = 1 - 3Y^{2}, \quad s_{2} = Y - 2Y^{2} + Y^{3}$$

 $s_3 = 3Y^2 - 2Y^3$ , Y = x/l

The kinetic energy of unit as follows:

$$Te = \frac{1}{2} \int_{0}^{1} dA \dot{r}^{T} \dot{r} dx = \frac{1}{2} \dot{e}^{T} m_{e} \dot{e}$$

Where d for beam density; A cross-sectional area for beam; Above variables ". " said partial derivatives of time; Quality of  $m_e$  as a unit,

$$m_{e} = \int_{0}^{l} \mathrm{d}AS^{\mathrm{T}}S\mathrm{d}x = \mathrm{d}Al\int_{0}^{l}S^{\mathrm{T}}S\mathrm{d}Y$$

Set f is the force per unit volume array, referred to as the physical and virtual work for physical strength is:  $\delta W = \int_0^l A W r^T f dx = \delta e^T Q_e$ 

The Q<sub>e</sub> as a unit generalized force,  $Q_e = \int_0^l AS^T f dx$ ,

According to the geometric nonlinear theory, the axial strain is

$$\mathbf{X}_{1} = \frac{1}{2} \left[ \left[ \frac{\partial r}{\partial x} \right]^{\mathrm{T}} \left[ \frac{\partial r}{\partial x} \right] - 1 \right]$$
(2)

 $S_1 = S'^T S' = \frac{1}{l^2} S_Y^T S_Y$ , The shape function of  $S_Y$  is S partial derivative matrix of Y. For axial deformation of beam element is:

 $U = -\frac{1}{2} \int_{0}^{l} E \Delta \mathbf{X}^{2} d\mathbf{x}$ 

$$U_{\rm e1} = \frac{1}{2} \int_0^1 EAX^2 dx$$

Then the stiffness matrix of and axial deformation can be written as

$$K_1 = \frac{1}{2} EAl \left[ \int_0^l \left( e^{\mathrm{T}} S_1 e \right) S_1 \mathrm{dY} - \int_0^l S_1 \mathrm{dY} \right]$$

Bending deformation for the beam element is

$$U_{\rm et} = \frac{1}{2} \int_0^l E I \kappa^2 \mathrm{d}x$$

Among them: *I* is cleansing and moment of inertia;  $\kappa$  for the deformation of back rest axis curve curvature, due to the axial deformation of beam usually is less than bending deformation, is a small amount, so it is assumed that the beam element in axial deformation of the constant  $\overline{f}$ , so

$$\kappa = \left| \frac{\mathrm{d}^2 r}{\mathrm{d}s^2} \right| = \frac{1}{\overline{f}} \sqrt{e^{\mathrm{T}} S''^{\mathrm{T}} S'' e} \tag{3}$$

On the type, the superscript "" "said x second order partial derivatives; the average deformation is

$$\overline{f} = \sqrt{\int_0^l f^2 d\mathbf{Y}} = \sqrt{\int_0^l r'^{\mathrm{T}} r' d\mathbf{Y}} = \sqrt{e^{\mathrm{T}} \overline{S}_{1e}}$$
  
So

$$U_{\rm et} = \frac{1}{2} \int_0^1 E I \kappa^2 dx = \frac{1}{2} e^{\rm T} K_{\rm t} e$$

Among them, the stiffness matrix of  $K_{\rm t}$  is related to the bending deformation,

$$K_{t} = EII \left[ \frac{1}{\overline{f}^{4}} \int_{0}^{l} S''^{T} S'' dY - \frac{2}{\overline{f}^{6}} e^{T} \left[ \int_{0}^{l} S''^{T} S'' dY \right] e\overline{S}_{1} \right]$$

The overall stiffness matrix of the  $k_e$  unit of beam is  $K_{ke} = K_1 + K_t$ .

A q for overall node coordinates array of single beam, n is beam unit number,  $B_{ke}$  for Boolean matrix of beam element, so  $e = B_{ke}q$ .

Among them, q for the 4 order array  $(4n+4)\times 1$ . Single beam, therefore, the overall quality of matrix, the elastic stiffness matrix and generalized force matrix, respectively are

$$m = \sum_{ke} B_{ke}^{\mathrm{T}} m_{ke} B_{ke}$$
$$K = \sum_{ke} B_{ke}^{\mathrm{T}} (K_{1} + K_{t}) B_{ke}$$
$$Q = \sum_{ke} B_{ke}^{\mathrm{T}} Q_{ke}$$

For N consisting of a beam system, upper right standard said beam label, for beam *i* dynamics equation is  $m^{(i)} \ddot{q}^{(i)} + K^{(i)} q^{(i)} = Q^{(i)}$   $i = 1, 2, \dots N$ 

#### 2. Beam System Dynamics Equation

To form a beam generalized coordinates of nodes in the system, mass matrix, the elastic stiffness matrix and generalized force matrix, the dynamic equation of free beam system is  $m\ddot{q} + Kq = Q$ , Among them:

$$m = \begin{bmatrix} m^{(1)} & 0 & \cdots & 0 \\ 0 & m^{(2)} & \cdots & 0 \\ \vdots & \vdots & \vdots & \vdots \\ 0 & 0 & \cdots & m^{(N)} \end{bmatrix}$$
$$K = \begin{bmatrix} k^{(1)} & 0 & \cdots & 0 \\ 0 & m^{(2)} & \cdots & 0 \\ \vdots & \vdots & \vdots & \vdots \\ 0 & 0 & \cdots & m^{(N)} \end{bmatrix}$$
$$Q = \begin{bmatrix} Q^{(1)} \\ Q^{(2)} \\ \vdots \\ Q^{(N)} \end{bmatrix} \qquad q = \begin{bmatrix} q^{(1)} \\ q^{(2)} \\ \vdots \\ q^{(N)} \end{bmatrix}$$

If with rotating hinge connections between the beam, and set up

$$q^{(i)} = \begin{bmatrix} e_1^{(i)} & e_2^{(i)} & \cdots & e_{4n+1}^{(i)} & \cdots & e_{4n+4}^{(i)} \end{bmatrix}^{\mathrm{T}}$$
  
The kinematic constraint equations of the beam between is  

$$e_{4n+1}^{(i)} = e_1^{(i+1)}, \qquad e_{4n+2}^{(i)} = e_2^{(i+1)}$$
  
 $i = 1, 2, \cdots N - 1$   

$$\begin{pmatrix} e_{4n+1}^{(i)} & e_{4n+2}^{(i)} \end{pmatrix}$$
 for the *i* beam and the right endpoint of absolute coordinates;  $\begin{pmatrix} e_{1}^{(i+1)} & e_{2}^{(i+1)} \end{pmatrix}$  for the *i*+1 girder left  
endpoint of the absolute coordinates. Then, the number of independent elements for Q is  
 $\overline{n} = (4n+4)N - 2(N-1) = (4n+2)N + 2$   
Assume that Q node coordinates W array for independence, can be expressed as

 $\bar{q} = \begin{bmatrix} \cdots & e_1^{(i)} & e_2^{(i)} & \cdots & e_{4n-1}^{(i)} & e_{4n}^{(i)} & e_{4n+3}^{(i)} & e_{4n+4}^{(i)} & e_1^{(i+1)} & e_2^{(i+1)} & \cdots \end{bmatrix}$ 

The relationship between q and q is q = Dq, Among them, D is the  $(4n+1)N \times \overline{n}$  order sparse array. The dynamic equation of beam system is

$$\overline{\overrightarrow{m} q} + \overline{K} \overline{q} = \overline{Q}$$
  
$$\overline{\overrightarrow{m}} = D^{\mathrm{T}} m D; \quad \overline{K} = D^{\mathrm{T}} K D; \quad \overline{Q} = D^{\mathrm{T}} Q.$$

#### 3. Numerical Examples

As shown in figure 2, flat beam by horizontal position without falling velocity, A side for rotating A, B side free. The parameters of the beam is: d=2766.67kg/m3, l=1.8 m, A=25cm3, E=68.95 GPa, I=0.130cm<sup>4</sup>, acceleration of gravity is 9.81 m/s2, the beam is divided into four units.

i



#### Fig 2: Single Pendulum

Left endpoint of the beam at A boundary condition for  $e_1^{(1)} = 0$ ,  $e_2^{(1)} = 0$ . In order to facilitate comparing with an approximate mixed coordinates method, the absolute displacement into the lateral deformation of the endpoint, Put  $\theta$  as beam rotation angle of floating coordinate  $O_e x_e y_e$  system,  $e_3^{(1)} \cdot e_4^{(1)}$  slope for A point.

Then

$$\sin\theta = \frac{e_4^{(1)}}{\sqrt{(e_3^{(1)})^2 + (e_4^{(1)})^2}}$$
$$\cos\theta = \frac{e_3^{(1)}}{\sqrt{(e_3^{(1)})^2 + (e_4^{(1)})^2}}$$

Set *A* is  $O_e x_e y_e$  coordinate system about the direction cosine matrix about the absolute coordinate system, type  $u_1$ ,  $u_2$  respectively, of the longitudinal and transverse deformation of beam end, then

$$A = \begin{bmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{bmatrix}$$
$$\begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = A^{\mathrm{T}} \begin{bmatrix} r_1(l) \\ r_2(l) \end{bmatrix} - \begin{bmatrix} l \\ 0 \end{bmatrix}$$

Figure 3 for this article is absolutely node coordinates method to calculate the direction of the beam end  $y_0$  absolute displacement. By figure 3 shows, compared with the absolute displacement due to deformation is small, take different modulus of elasticity, the absolute displacement basic same, explain the influence of the elastic deformation of absolute displacement is smaller. Figure 4 is absolutely node coordinate method and an approximate mixed coordinates method to calculate the lateral deformation of the beam of the endpoints. By figure can be seen, when E = 6.895GPa, due to the lateral deformation is small, the paper definitely node coordinate method and an approximate mixed coordinates method to calculate the lateral deformation of the beam of the beam of the endpoints. By figure can be seen, when E = 6.895GPa, due to the lateral deformation is small, the paper definitely node coordinate method and an approximate mixed coordinates method to calculate the lateral deformation of the beam of the beam of the endpoints; the maximum lateral deformation when E = 6.895GPa and E = 68.95GPa are 10 times, in line with the expected results, and the lateral deformation increase. An approximation of the hybrid coordinate method due to ignore the deformation in dynamic equation of higher order item, lateral deformation from the start because cannot satisfy the calculation accuracy and numerical divergence, absolute coordinates method due to the dynamics in this paper considering the deformation of high order term in the equation, the lateral deformation of beam endpoint convergence, shows the tendency of up and down shocks.



Fig 3: Absolute Displacement of the Beam tip in y<sub>0</sub> Direction





In order to further verify the results, the energy analysis. The system provides a conservative system, in the process of swing beam, should accord with energy conservation, namely the total kinetic energy, elastic deformation energy, the conservation of the sum of the gravitational potential energy. In this example, the initial position, and take the overall coordinate system origin at point A so total energy conservation should be zero. That is

$$T + U + U_{a} = 0$$

In the type, T is kinetic energy;  $U_g$  is the gravitational potential energy; U is elastic deformation energy. Figure 5 for energy curve of absolute coordinates method by the chart shows, in this paper, absolute coordinates method to calculate the total energy constant is zero. Conform to the law of conservation of energy, the absolute node coordinates method is proved to be correct. When E = 6.895GPa total energy remains zero, thus large deformation case absolute value of node coordinates method is accurate.



In small deformation, with an approximate mixed coordinates method generalized coordinates will be cut by modal truncation method, computational efficiency is higher. In this example, an approximate mixed coordinates method for calculation of 10, absolute coordinates method of calculation time was 80, so in the case of small deformation should try to use a similar to the hybrid coordinate method. Example 2. By the horizontal position without falling velocity of the flexible double pendulum, two beams of physical parameters are the same as the case 1, E = 68.95GPa .Figure 6 for point *B* the  $y_0$  direction absolute displacement, figure 7 for the system energy curve.By the figure shows, sports system in the process of the total energy constant is zero, conform to the law of conservation of energy, can attest to the absolute node coordinates method in the flexible multi-body system dynamics problem has higher calculation accuracy.







Fig 7: Energy of the Beam System

## 4. Conclusions

- (1) A similar mixed coordinates method is only suitable for small deformation condition.
- (2) Absolute node coordinates method is suitable for the small deformation condition, and is suitable for large deformation, and the result is quite accurate.
- (3) In small deformation, with an approximate mixed coordinates method generalized coordinates will be cut by modal truncation method, computational efficiency is higher, so in the case of small deformation should try to use a similar to the hybrid coordinate method.

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